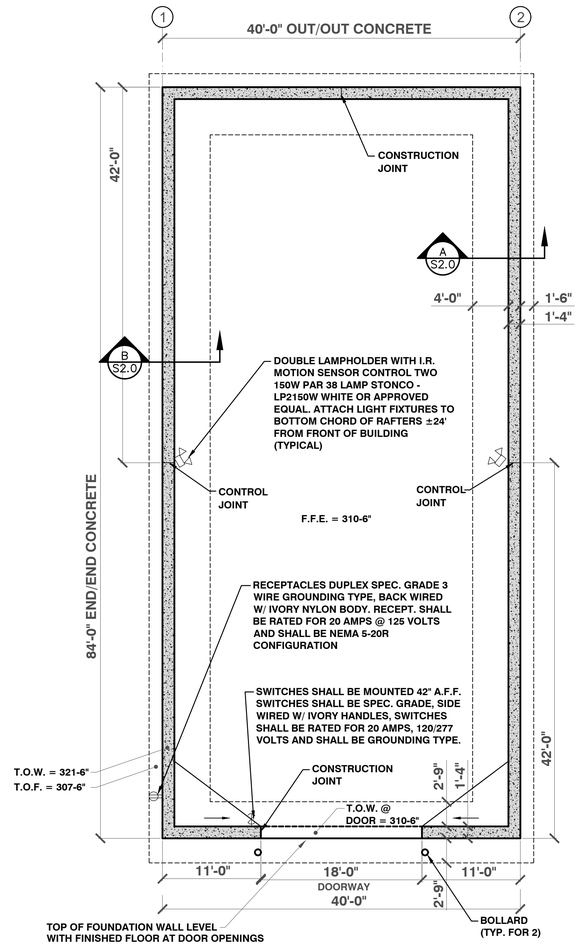
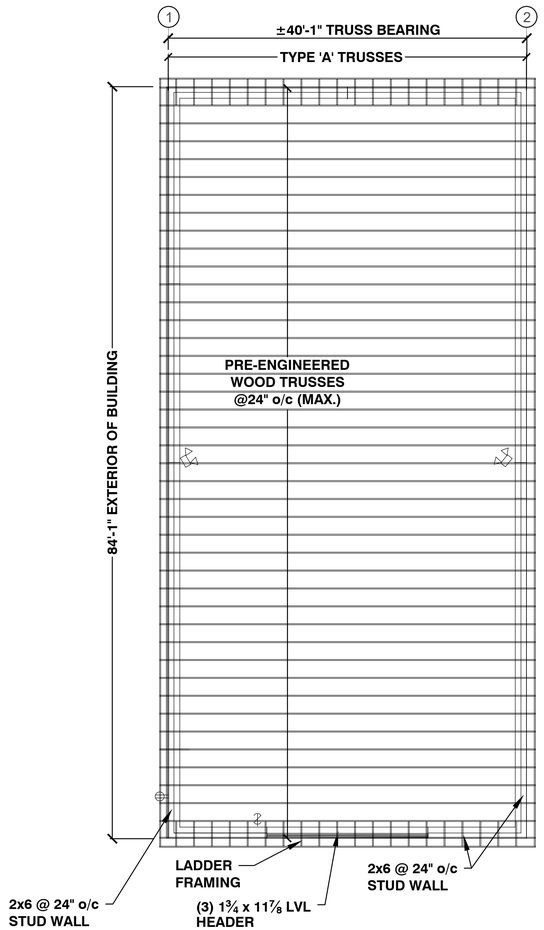


FOUNDATION/FLOOR PLAN

1/8" = 1'-0"



ROOF FRAMING PLAN

1/8" = 1'-0"

GENERAL NOTES

1. All work shall be in accordance with the (IBC) International Building Code, 2015 edition, and the requirements of all local building codes.
2. The Contractor shall field verify all critical dimensions before fabrication of any materials.
3. All expansion bolts shall be as manufactured by Hilti, or approved equal. (See drawings for size and type.) Install in accordance with manufacturer's recommendations.
4. The structural drawings herein represent the finished structure. The Contractor shall provide all temporary guying and bracing required to erect and hold the structure in proper alignment until all structural work and connections have been completed. The investigation, design, safety, adequacy and inspection of erection bracing, shoring, temporary supports, etc. is the sole responsibility of the Contractor.
5. The Engineer shall not be responsible for the methods, techniques and sequences of the procedures to perform the work. The supervision of the work is the sole responsibility of the Contractor.
6. Contractors shall visit the site prior to bid to ascertain conditions which may adversely affect the work or cost thereof.
7. All timbers in contact w/concrete or masonry shall be pressure treated.
8. Connections in contact with treated wood shall be installed in accordance with manufacturers recommendations which may require connectors with increased galvanizing or stainless steel connectors.
9. Shop drawings and other items shall be submitted to the Engineer for review prior to fabrication. All shop drawings shall be reviewed by the General Contractor before submittal. The Engineer's review is to be for conformance with the design concept and general compliance with the relevant Contract Documents. The Engineer's review does not relieve the Contractor of the sole responsibility to review, check and coordinate the shop drawings prior to submission. The Contractor remains solely responsible for errors and omissions associated with the preparation of shop drawings as they pertain to member sizes, details, dimensions, etc.

GEOTECHNICAL NOTES

1. All organics, topsoil, stratified silty sands and asphalt shall be stripped from the site prior to placement of any structural fill material.
2. Excavation and preparation of the subgrade materials, structural fill placement and compaction below slab should only occur during fair weather periods to ensure that the subgrade materials remain unsaturated.
3. Preparation of the footing subgrade shall be performed by excavation of the native soil with a smooth bladed bucket. No footing excavation or preparation should take place during any excessive rain events.
6. Footing and subgrade conditions should be inspected prior to placement of any structural fill or footings. Inspections should be performed by a qualified geotechnical engineer licensed in the State of Vermont.
7. Footing subgrade preparation:
   - Non-ledge locations: Footings to bear on undisturbed, firm natural soil or 8" compacted drainage stone
   - Ledge locations: Chink the surface of the blasted rock with an 12" (min.) layer of Dense Graded Crushed Stone (Vtrans 704.06) and compact to at least 95% of the Modified Proctor value (ASTM D-1557)
8. Care should be taken during construction to divert surface water away from open excavations as damp or saturated soils may destabilize the subgrade conditions or make proper compaction difficult.
9. If subgrade disturbance occurs, the disturbed material should be over-excavated and replaced with separation fabric and compacted crushed stone or low strength flowable fill.
10. Temporary dewatering will be required during construction to prevent groundwater from ponding on the exposed subgrade.
11. All final bearing grades should be firm, stable and free of loose soil, mud, water, frost or other deleterious materials.

FOUNDATION NOTES

1. All footings have been designed based upon a max. soil bearing pressure of 3,000 psf. All footings shall bear on undisturbed, firm natural soil or compacted fill in accordance with Geotechnical notes.
2. No unbalanced backfilling shall be done against foundation walls unless walls are securely braced against overturning, either by temporary bracing or by permanent construction.

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DRAWN: PJM
CHECKED: BCE
APPROVED: BCE

OWNER:

STATE OF VERMONT
VERMONT AGENCY OF TRANSPORTATION
MONTPELIER, VERMONT

PROJECT:

DISTRICT #5
NEW HAVEN SALT SHED

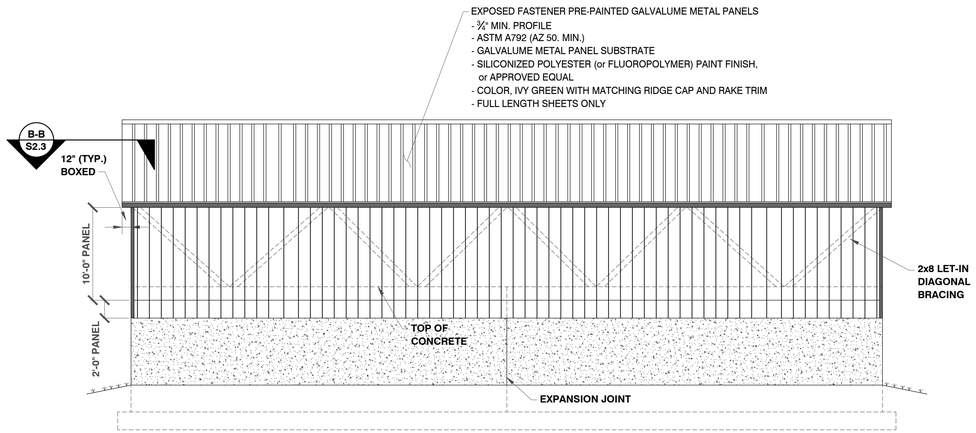
490 MAIN STREET
NEW HAVEN, VT

DATE	CHECKED	REVISION
2/26/24	BCE	BID DOCUMENTS

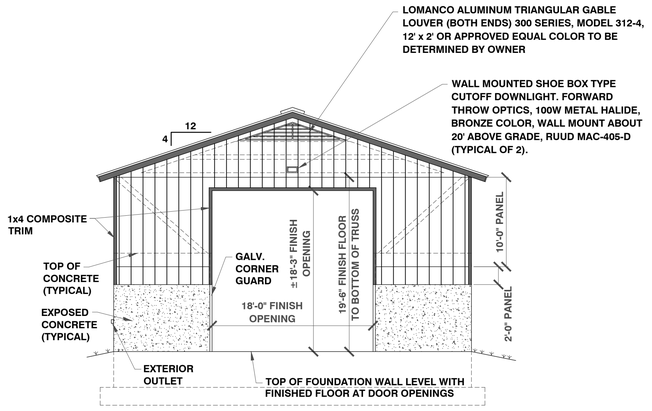
FOUNDATION/ FRAMING PLANS and NOTES

DATE: 2/26/2024
SCALE: AS SHOWN
PROJ. NO.: 22198.01

DRAWING NUMBER: S1.0

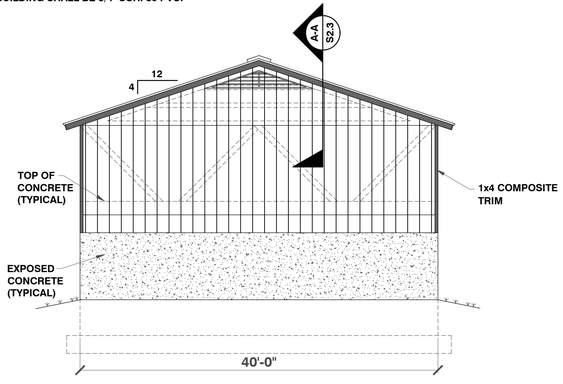


SIDE ELEVATION
1/8" = 1'-0"



FRONT ELEVATION
1/8" = 1'-0"

NOTE:
ALL ELECTRICAL CONDUIT BELOW GRADE AND ON THE EXTERIOR OF THE BUILDING SHALL BE 1" RIGID CONDUIT. CONDUIT INSIDE THE BUILDING SHALL BE 3/4" SCH. 80 PVC.



BACK ELEVATION
1/8" = 1'-0"

GENERAL BUILDING NOTES

TRUSS NOTES:

TRUSS MANUFACTURER TO PROVIDE ENGINEER A SET OF ENGINEERED & STAMPED DRAWINGS FOR APPROVAL PRIOR TO TRUSS MANUFACTURE (ENGINEER MUST BE LICENSED IN VERMONT).

TRUSS MANUFACTURER TO PROVIDE THE CONTRACTOR WITH ALL NECESSARY INSTRUCTIONS FOR PROPER TRUSS INSTALLATION, ANCHORAGE, BRACING.

LUMBER NOTES:

FRAMING MEMBERS TO BE KILN DRIED CONSTRUCTION GRADE SPRUCE S4S 19% MOISTURE CONTENT.

PLATES & NAILERS IN CONTACT WITH CONCRETE TO BE CCA 0.40 TREATED.

ALL FASTENERS TO BE GALVANIZED AND SHALL BE APPROPRIATELY SIZED.

FINISH NOTES:

ROOFING COLOR: IVY GREEN (BY AMERI-DRAIN) OR AS DIRECTED BY ENGINEER

PAINT TO BE OLYMPIC ACRYLIC LATEX SATIN FINISH. COLOR TO BE DETERMINED BY OWNER/ENGINEER.

APPLY PAINT AS SOON AS POSSIBLE AFTER SIDING IS INSTALLED.

PRE-PRIME ALL FINISH MATERIAL PRIOR TO INSTALLATION.

APPLY TWO COATS OF FINISH.

CONCRETE NOTES:

RE-BAR TO BE CONTINUOUS THROUGH CONSTRUCTION JOINTS.

1/2" "V" JOINTS IN EXPOSED FACE OF CONSTRUCTION JOINTS.

3/4" CHAMFER ON EACH FACE OF EXPANSION JOINTS.

NO HORIZONTAL CONSTRUCTION JOINTS IN WALLS – CONTINUOUS POUR.

CONTRACTOR TO LEAVE NO METAL WITHIN 1" OF SURFACE. USE 1" DEEP PLASTIC CONES AT ALL TIES AND PLUG ALL TIE HOLES WITH AN APPROVED NON-SHRINK GROUT.

ALL ROUGH CONCRETE FORM JOINTS AND OTHER ROUGH SURFACES TO BE RUBBED FINISH.

ALL RE-BAR TO BE GRADE 60 WITH 3" CLEARANCE ON ALL SIDES UNLESS NOTED OTHERWISE. RE-BAR TO BE STORED ON BLOCKING 4" MINIMUM ABOVE GROUND.

SMARTSIDE 190 SERIES $19/32$"(0.578") TREATED ENGINEERED WOOD PANEL SIDING

MATERIAL: EXTERIOR-GRADE PHENOLIC RESIN-SATURATED PAPER OVERLAY LAMINATED TO EPA-REGISTERED ZINC-BORATE-PRESERVATIVE-TREATED ENGINEERED WOOD SIDING. APA PRP-108 RATED, AWPA COMPLIANT, PRIMED FOR PAINTING.

48" WIDE PANELS, SHIPLAP EDGES, CHANNEL GROOVES AT 8 INCHES ON CENTER, ½" THICK

EMBOSSED ROUGH SAWN CEDAR SURFACE

WARRANTY: 5 YEAR FINISH/50 YEAR SUBSTRATE

FINISH: PRIME ALL EXPOSED FIELD-CUT EDGES WITH A HIGH QUALITY EXTERIOR PRIMER FORMULATED FOR USE ON ENGINEERED WOOD SUBSTRATES.

APPLY 2 COATS, EXTERIOR GRADE ACRYLIC LATEX PAINT, SPECIALLY FORMULATED FOR USE ON WOOD AND ENGINEERED WOOD SUBSTRATES.

COLOR TO BE DETERMINED BY OWNER

WOOD SIDING INSTALLATION NOTES:

8d GALVANIZED NAILS FOR 190 SERIES $19/32$" (0.578") SMARTSIDE PANELS PENETRATING STUDS 1½"

NAIL SPACING: 6" MAX. PERIMETER, 12" MAX FIELD, 3/8" MIN. FROM EDGES. NAILS TO BE SET FLUSH. DO NOT COUNTERSINK NAILS INTO PANEL.

MINIMUM 3/8" GAP REQUIRED AT HORIZONTAL JOINTS OF PANEL SIDING.

SMARTSIDE PANEL SIDING MUST NOT CONTACT CONCRETE. 1/2" CLEARANCE MIN.

SMARTSIDE PANEL SIDING MUST NOT CONTACT GROUND. 6" CLEARANCE MIN.

PAINT ALL PANEL SURFACES INCLUDING BOTTOM EDGES

ADDITIONAL SMARTSIDE PANEL INSTALLATION INFO CAN BE FOUND AT WWW.SMARTSIDEONLINE.COM OR CALL 800-648-6893

INSTALL IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS

TREATED EXTERIOR COMPOSITE TRIM/SOFFIT

MATERIAL: TREATED EXTERIOR COMPOSITE

TREATMENT: ZINC BORATE

WARRANTY: 5 YEARS PRIMER/30 YEARS SUBSTRATE

CONSTRUCTION: ONE SOLID PIECE, NOT LAMINATED, UNIFORM DENSITY, TREATED WOOD FIBER

SURFACE: CLEAR WOOD GRAIN TEXTURE ON ONE SIDE, SMOOTH ON THE OTHER

COATING: FACTORY PRIMED ON FOUR SIDES WITH A MILDEW RESISTANT PRIMER

FINISH: PRIME ALL EXPOSED FIELD-CUT EDGES WITH A HIGH QUALITY EXTERIOR PRIMER FORMULATED FOR USE ON COMPOSITE WOOD.

APPLY 2 COATS, EXTERIOR GRADE ACRYLIC LATEX PAINT, RECOMMENDED BY THE PAINT MANUFACTURER FOR APPLICATION OVER COMPOSITE WOOD SUBSTRATES

COLOR TO BE DETERMINED BY OWNER

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BUILDING ELEVATIONS and NOTES

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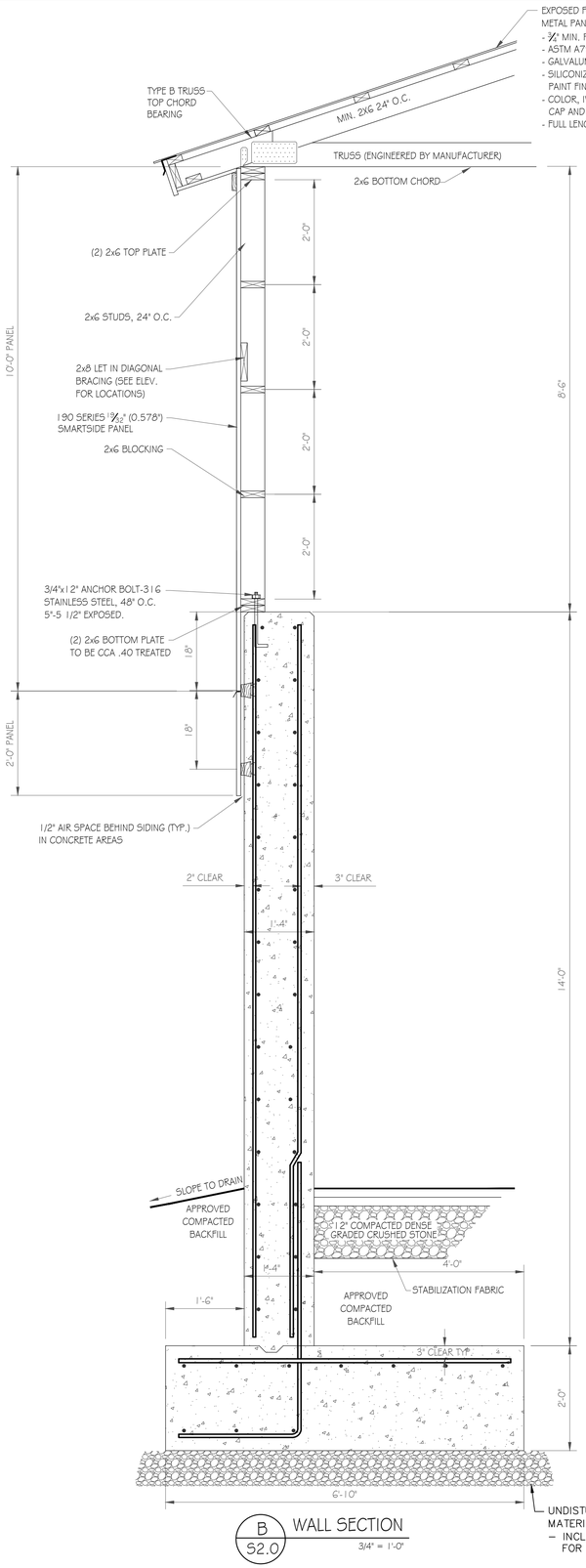

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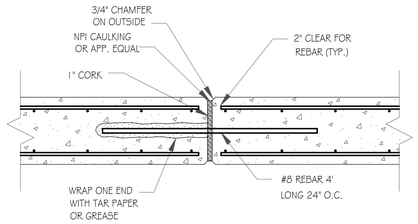
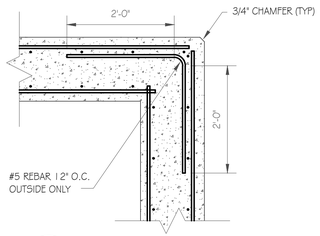
WALL SECTIONS

DATE 2/26/2024
SCALE AS SHOWN
PROJ. NO. 22198.01
DRAWING NUMBER S2.0



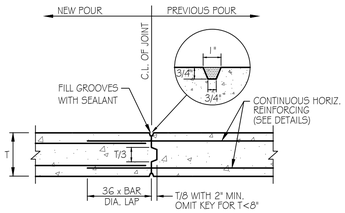
1 / S2.1 EXPANSION JOINT DETAIL

3/4" = 1'-0"



4 / S2.1 CORNER DETAIL

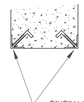
3/4" = 1'-0"



NOTE: Construction joints shall be placed as shown on the plans. No horizontal joints shall be permitted. All foundation walls shall be adequately braced to withstand earth and construction load pressures.

2 / S2.1 WALL CONSTRUCTION JOINT

N.T.S.

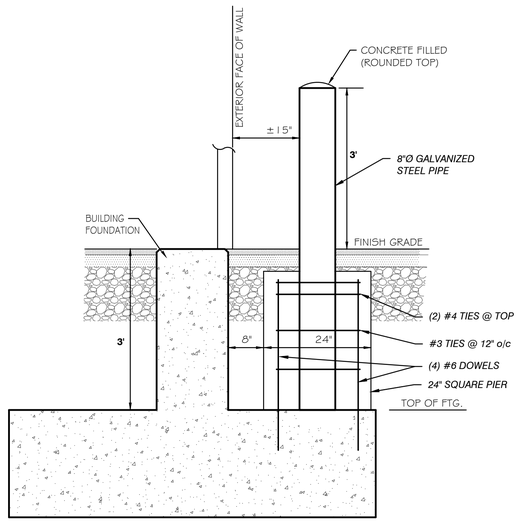


3"X3"X1/4" ANGLE IRON (A-36 STEEL) WITH 1/2"ØX6" LONG ANCHOR BOLTS WELDED 18" O.C. FULL CONC. HEIGHT. HOT DIP GALV. AFTER FABRICATION, PRIOR TO INSTALLATION.

CORNER GUARD TO EXTEND FROM FLOOR TO TOP OF CONCRETE WALL.

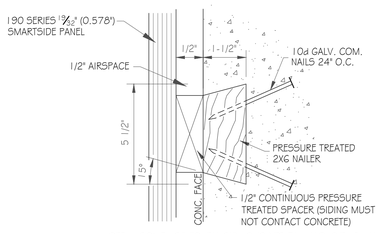
5 / S2.1 CORNER DETAIL AT OPENINGS

3/4" = 1'-0"



3 / S2.1 BOLLARD DETAIL

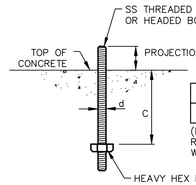
N.T.S.



NOTE: SOME HORIZONTAL SIDING JOINTS FALL ALONG THIS NAILER. LOCATION OF NAILER IN CONCRETE IS CRITICAL.

6 / S2.1 NAILER DETAIL

N.T.S.



SIZE	1/2"	5/8"	3/4"	1"	1 1/4"	1 1/2"
C	6"	8"	10"	14"	18"	20"

(MINIMUM LENGTHS SHOWN) ANCHOR RODS SHALL BE ASTM F1554, GRADE 55 WITH COMPATIBLE NUTS & WASHERS

7 / S2.1 ANCHOR ROD SCHEDULE

N.T.S.

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FOUNDATION DETAILS

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DRAWING NUMBER: S2.1

CAST-IN-PLACE CONCRETE

1. Concrete shall be composed of Type I Portland Cement conforming to ASTM C150, fine and coarse aggregates conforming to ASTM C33, and mixing water free of oil, acid, or injurious amounts of alkalis and other salts. Air entraining admixtures shall conform to ASTM C260.

2. Concrete shall conform to the following compressive strength and slump requirements:

Concrete Location	Min. f'c (28 days)	Slump*	Air Entrainment	max w/c
Footings	4,000 psi	4" max.	Not Required	0.45
Foundation walls/Columns/Piers	4,000 psi	4" max.	Required (7% ±1.5)	0.45

* At Contractor's option, an approved admixture may be used to produce flowable concrete. Maximum slump shall not exceed 7 inches.

3. Chemical Admixtures: If used, provide admixtures certified by manufacturer to be compatible with other admixtures and to contain not more than 0.1 percent water–soluble chloride ions by mass of cementitious material.
A. Water–Reducing Admixture: ASTM C494, Type A.
B. Retarding Admixture: ASTM C494, Type B.
C. Water–Reducing and Retarding Admixture: ASTM C494, Type D.
D. High–Range, Water–Reducing Admixture: ASTM C494, Type F.
E. High–Range, Water–Reducing and Retarding Admixture: ASTM C494, Type G.
F. Plasticizing and Retarding Admixture: ASTM C1017, Type II.
G. Corrosion–Inhibiting Admixture: Commercially formulated, anodic inhibitor or mixed cathodic and anodic inhibitor; capable of forming a protective barrier and minimizing chloride reactions with steel reinforcement in concrete and complying with ASTM C494/C494M,type C.

4. All concrete work shall conform to the requirements of ACI 301, "Specifications for Structural Concrete Buildings". Hot weather concreting shall be in accordance with ACI 305. Cold weather concreting shall be in accordance with ACI 306.

5. All reinforcing steel shall be set and tied in place prior to pouring of concrete, except that vertical dowels for masonry wall reinforcing may be "floated" in place. Do not field bend bars partially embedded in hardened concrete unless specifically indicated or approved by the Engineer. Support for reinforcing shall be via protected metal spacers, chairs, bolsters, or ties.

6. All edges of permanently exposed concrete surfaces shall be chamfered 3/4" unless otherwise noted.

7. Protect newly placed concrete against low and high temperature effects and against rapid loss of moisture. Cure all concrete for at least seven (7) days at a temperature of at least 50 degrees Fahrenheit by approved curing methods. Forms may be stripped when the concrete has attained sufficient strength to carry its own weight and any applied loads.

8. Use corrosion–inhibiting admixture in concrete admixtures, not required in footings. – Use dosage rate of 3 gallons/cubic yard.

CONCRETE FIELD QUALITY CONTROL

1. Testing Agency: Contractor shall obtain a qualified independent testing and inspecting agency to perform field tests and inspections and prepare test reports.

2. Testing Services: Testing of composite samples of fresh concrete obtained according to ASTM C 172 shall be performed according to the following requirements:

   A. Testing Frequency: Obtain at least 1 composite sample for each 50 cu. yd. or fraction thereof of each concrete mix placed each day.

   B. Slump: ASTM C 143; one test at point of placement for each load, but not less than one test for each day's pour of each concrete mix. Perform additional tests when concrete consistency appears to change.

   C. Air Content: ASTM C 231, pressure method; one test for each load, but not less than one test for each day's pour of each concrete mix.

   D. Concrete Temperature: ASTM C 1064; one test hourly when air temperature is 40 deg F and below and when 80 deg F and above, and one test for each load.

   E. Compression Test Specimens: ASTM C 31; cast and laboratory cure one set of four standard cylinder specimens for each composite sample.

   F. Compressive–Strength Tests: ASTM C 39; test 1 specimen at 7 days and 2 specimens at 28 days. Retain one specimen for a 56 day test in the event the 28 day tests do not meet the specifications.
      i. A compressive–strength test shall be the average compressive strength from 2 specimens obtained from same composite sample and tested at 28 days.

3. Strength of each concrete mix will be satisfactory if average of any 3 consecutive compressive–strength tests equals or exceeds specified compressive strength and no compressive–strength test value falls below specified compressive strength by more than 500 psi.

4. Test results shall be reported in writing to Architect/Engineer, concrete manufacturer, and Contractor within 48 hours of testing. Reports of compressive–strength tests shall contain Project identification name and number, date of concrete placement, name of concrete testing and inspecting agency, location of concrete batch in Work, design compressive strength at 28 days, concrete mixture proportions and materials, compressive breaking strength, and type of break for both 7– and 28–day tests.

REINFORCING STEEL

1. Typical
   A. Reinforcing bars shall be ASTM A615–Grade 60 and shall be detailed, fabricated and installed in accordance with the latest A.C.I. specifications.
   B. Welded wire mesh (W.W.M.) shall be ASTM A185. Lap all splices 12" minimum. Securely fasten W.W.M. in place to prevent movement during concrete placement.

2. Epoxy coated (where noted on plans)
   A. Epoxy–Coated Reinforcing Bars: ASTM A775/A775m or ASTM A934/A934M; with ASTM A615/A615M, Grade 60 deformed bars.
   B. Epoxy–Coated Welded–Wire Reinforcement: ASTM A884/A884M, Class A, plain steel.
   C. Epoxy Repair Coating: Liquid, two–part, epoxy repair coating, compatible with epoxy coating on reinforcement.

3. Galvanized (where noted on plans)
   A. Galvanized Reinforcing Bars: ASTM A767/A767M or ASTM A1094/A1094M; with ASTM A615/A615M, Grade 60 deformed bars.
   B. Galvanized Welded–Wire Reinforcement: ASTM A1060/A1060M, Class A, plain steel
   C. Coating Repair: In accordance with ASTM A780/A780M.

4. All horizontal rods are continuous. Lap all splices 30 diameters unless otherwise noted. Provide corner bars and dowel into existing walls. 4. Provide a clear cover from reinforcing steel to adjacent concrete surfaces as follows:
   A. Concrete cast against earth: 3"
   B. Formed concrete exposed to earth or weather:
      #5 and smaller – 1 1/2"
      #6 and larger – 2"
   C. Formed concrete not exposed to earth or weather:
      Slabs, walls – 3/4"

RIGID INSULATION

Extruded–polystyrene board insulation: ASTM C 578, of type and density indicated below, with maximum flame–spread and smoke–developed indexes of 75 and 450, respectively:

1. Available manufacturers:
   a. DiversiFoam Products.
   b. Dow Chemical Company
   c. Owens Corning
   d. Pactiv Building Products Division
2. Type IV, 1.60 lb/cu. ft. minimum density, 25.0 psi compressive strength
3. Type VI, 1.80 lb/cu. ft. minimum density, 40.0 psi compressive strength
4. Type VII, 2.20 lb/cu. ft. minimum density, 60.0 psi compressive strength
5. Type V, 3.00 lb/cu. ft. minimum density, 100.0 psi compressive strength

ROUGH CARPENTRY

1. Submittals: Statement of species and grade selected for each application. Lumber to include visible grade stamp of agency certified by National Forest Products Association (NFPA).

2. Materials
   A. Lumber: Any species listed in National Design Specification Supplement. Maximum moisture content of 19 percent of following grades:
      i. Structural Light Framing: No. 2 grade or better.

      ii. Typical Lumber shall be capable of meeting the Spruce Pine Fir design values, and where indicated in the plans, Douglas Fir shall be used:

Spruce Pine Fir	Douglas Fir
Fb = 875 psi	Fb = 900 psi
Fv = 135 psi	Fv = 180 psi
$F_{c\perp}$ = 1,150 psi	$F_{c\perp}$ = 1,350 psi
$F_{c\perp}$ = 425 psi	$F_{c\perp}$ = 625 psi
E = 1,400,000 psi	E = 1,600,000 psi

   B. Prefabricated Laminated Veneer Lumber (LVL) or Parallel Strand Lumber (PSL) headers and beams: LVL material shall be 1.9E, Southern Pine, and PSL material shall be 2.0, Southern Pine. Do not cut or notch material without the Manufacturer's approval. Lumber shall be capable of meeting the following design values:

Laminated Veneer Lumber (LVL)	Parallel Strand Lumber (PSL)
Fb = 2600 psi (for 12" depth)	Fb = 2900 psi (for 12" depth)
Fv = 285 psi	Fv = 290 psi
$F_{c\parallel}$ = 2510 psi	$F_{c\parallel}$ = 2900 psi
$F_{c\perp}$ = 750 psi	$F_{c\perp}$ = 650 psi
E = 1,900,000 psi	E = 2,000,000 psi

   C. Fasteners for Preservative Treated Wood: Hot–dip galvanized steel (ASTM A153) or Stainless Steel as recommended by manufacturer.
   D. Nails, Spikes and Staples: Galvanized for exterior locations, high humidity locations and treated wood; plain finish for other interior locations; size and type to suit application.
   E. Bolts, Nuts, Washers, Lags, Pins and Screws: Medium carbon steel sized to suit application galvanized for exterior locations, high humidity locations and treated wood; plain finish for other interior locations.
   F. Joist Hangers: Sized and profiled to suit application; galvanized finish.
   G. Fasteners: Expansion shield and lag bolt type for anchorage to concrete. Bolts or power activated type for anchorage to steel.
   H. Pressure Treated Lumber
      i. Pressure treat boards and dimension lumber with waterborne preservative according to AWPA U1–07, Use Category UC3B (Exterior Construction, Above Ground, Uncoated).
      ii. Pressure treat timber with waterborne preservative according to AWPA U1–07, Use Category UC4A (Ground Contact, Non–critical componenets).

3. Structural steel plate connectors shall conform to ASTM A–36 specifications and be 1/4" thick unless otherwise indicated. Bolts connecting wood members shall be per ASTM A–307 and be 3/4" diameter unless otherwise indicated. Provide washers for all bolt heads and nuts in contact with wood surfaces.

4. Bolt holes shall be carefully centered and drilled not more than 1/16" larger than the bolt diameter. Bolted connections shall be snugged tight but not to the extent of crushing wood under washers.

5. All metal joist hangers, hurricane clips, hold–down anchors and other accessories shall be prefabricated. Install all accessories per the Manufacturer's requirements. All steel shall have a minimum thickness of 0.04 inches (per ASTM A446, Grade A). Galvanized hot–dipped (Coating G60) accessories shall be used at interior locations where Stainless Steel is not indicated. Stainless Steel (ASTM A666) shall be used at exterior locations unless indicated otherwise.

6. Beams, joists, rafters or studs shall not be notched, cut, or pierced in excess of the limitations permitted herein, unless suitably reinforced to transmit all calculated loads. Notches located at ends of the member shall not exceed 1/4 the depth. Notches not at ends shall not exceed 1/6 the depth of the members and shall not be located in the middle 1/3 of the span. Holes bored or cut into joists shall not be closer than 2 inches to the top or bottom of the joist and the diameter of the hole shall not exceed 1/3 the depth of the joist. In studs, notched or bored holes shall not be more than 1/3 the depth of the stud. When the stud is cut or bored in excess of 1/3 its depth, it shall be reinforced to be equal to the load carrying capacity of a stud notched or bored no more than 1/3 its depth.

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VERMONT AGENCY OF TRANSPORTATION
MONTPELIER, VERMONT

PROJECT:
DISTRICT #5
NEW HAVEN SALT SHED
490 MAIN STREET
NEW HAVEN, VT

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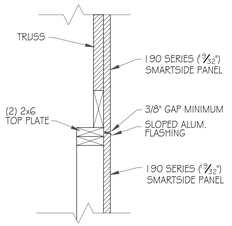
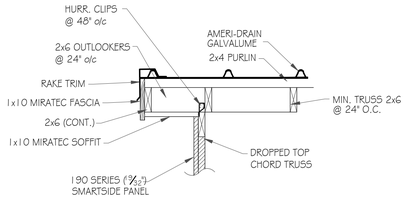
FOUNDATION & BACKFILL NOTES

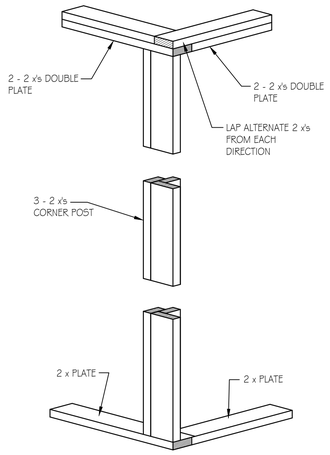
DATE: 2/26/2024
SCALE: AS SHOWN
PROJ. NO.: 22198.01
DRAWING NUMBER: S2.2



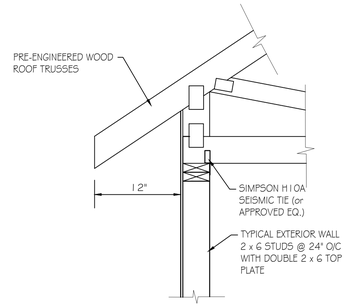
A-A SECTION
S2.3
1" = 1'-0"



B-B SECTION
S2.3
1" = 1'-0"

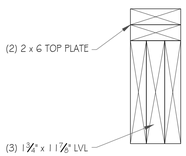


4 STUD WALL CORNER
S2.3
N.T.S.



TYPICAL TRUSS BEARING ON
STUD WALL
6
S2.3
N.T.S.

1 SEISMIC TIE DETAIL
S2.3
(MINIMUM DIMENSIONS SHOWN) N.T.S.



2 HEADER DETAIL
S2.3
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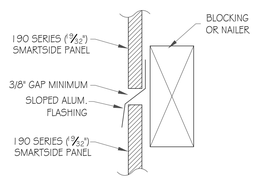
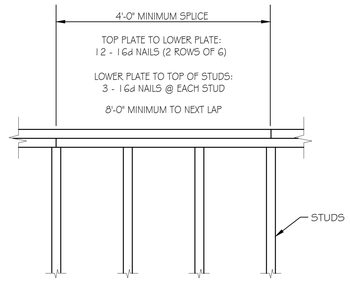
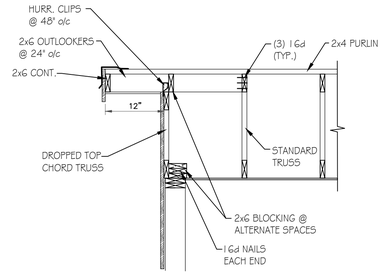


3 HORIZ. JOINT DETAIL
S2.3
N.T.S.



STUD WALL DOUBLE
PLATE SPLICE
5
S2.3
N.T.S.



7 ENDWALL SECTION
S2.3
N.T.S.

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FRAMING DETAILS

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22198.01

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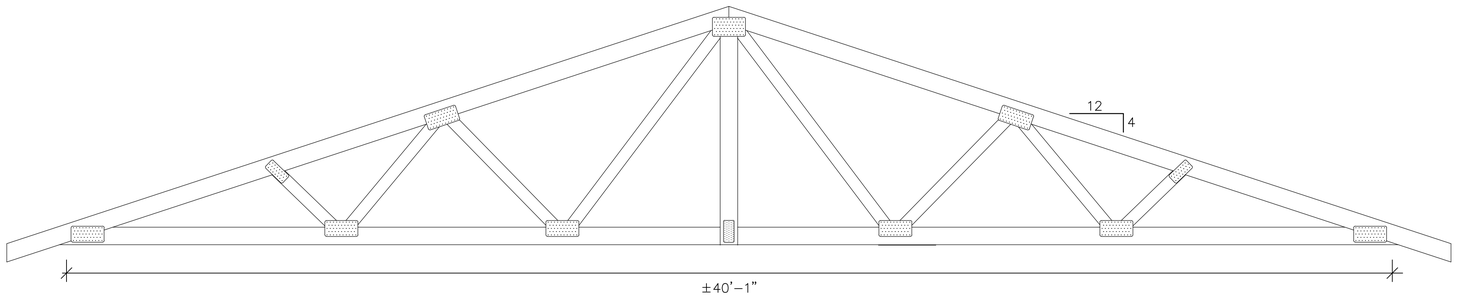

TRUSS 'A'

DESIGN NOTES:
1. TOP & BOTTOM CHORDS 2x6 (MIN.) INTERIOR CHORDS, 2x4 (MIN.) (UNLESS NOTED OTHERWISE)
2. TRUSSES @ 24" o/c (MAX.)

PRE-ENGINEERED WOOD TRUSSES

1. Wood trusses shall be designed by the Manufacturer to support the following loads:
   A. Gravity Loading Case
   – Top Chord Loading
   Live Load – Ground Snow Load, Pg = 40 psf (on the horizontal projection)
   – Include drift and sliding snow loads
   – The resultant flat roof snow load of Pf shall not be less than 40 psf.
   Dead Load – 15 psf (on the surface area)
   – Additional 5 psf at built-up framing areas

   – Bottom Chord Loading
   Attic Live Load – 20 psf (per IBC 2015)
   Dead Load – 10 psf

   B. Wind Loading Case
   – Per IBC 2015
2. Top and bottom chords shall be 2 x 6 minimum, and interior members shall be 2 x 4 minimum size.
3. Wood trusses shall be designed by the Manufacturer in accordance with the applicable provisions of the latest edition of the National Design Specification of the National Forest Products Association, and the Design Specification for Metal Plate Connected Wood Trusses of the Truss Plate Institute.
4. Wood materials shall be as designated in the National Design Specification and shall be kiln dried and used at 19% maximum moisture content. Provide Grade No. 2 or as required to satisfy stress requirements.
5. Wood trusses shall be erected in accordance with the truss Manufacturer's requirements. This work shall be done by a qualified and experienced Contractor.
6. The Contractor shall provide all temporary and permanent bracing as required for safe erection and performance of the trusses. The guidelines set forth by the Truss Plate Institute Publication "HIB-91, Commentary and Recommendations for Handling, Installing and Bracing Metal Plate Connected Wood Trusses" shall be a minimum requirement.
7. Truss members and components shall not be cut, notched, drilled nor otherwise altered in any way without the written approval of the Manufacturer.
8. Submit complete shop drawings for all wood trusses showing member sizes, species, grade, moisture content, span, camber, dimensions, chord pitch, bracing requirements and loadings. Shop drawings shall be submitted to the Engineer and shall bear the seal of a Professional Engineer registered in Vermont.

PLYWOOD/GYPBOARD SHEATHING

1. All plywood construction shall be in accordance with The American Plywood Association (APA) specifications.
2. All roof panel sheathing shall be 5/8" APA rated sheathing, 40/20, Exposure 1. Suitable edge support shall be provided by use of panel clips or blocking between framing. Unless otherwise noted connect roof sheathing with 8d common nails at 6" o/c at supported panel edges at 6" o/c at intermediate supports.
3. All floor sheathing shall be APA rated Sturd-I-Floor, Exposure 1 with tongue and groove edge. Unless otherwise noted connect floor sheathing with 10d common nails spaced 6" o/c at supported edges and 12" o/c at intermediate supports. Field-glue using adhesives meeting APA Specification AFG0-01, applied in accordance with the Manufacturer's recommendations.
4. All wall panel sheathing shall be 1/2" APA rated sheathing, 24/16, Exposure 1. Unless otherwise indicated, connect wall sheathing with 10d common nails spaced 6" o/c at supported panel edges and 12" o/c at intermediate supports.
5. Install all plywood sheathing with the long dimension of the panel across supports and with panel continuous over two or more spans. Stagger panel end joints. Allow 1/8" spacing at panel ends and edges unless otherwise recommended by the sheathing manufacturer.
6. All nailing shall be carefully driven and not overdriven. The use of staples is prohibited.

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TRUSS SECTIONS & NOTES

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DRAWING NUMBER: S2.4